ADDENDUM # 1

From: **Lena L. Butler, Purchasing Supervisor**

To: **Bidders**

Project: **Castle Hayne Park Improvements-RFB # 15-0185**

Date: **November 19, 2014**

This addendum is issued regarding questions received in response to "RFP # 15-0185-CASTLE HAYNE PARK IMPROVEMENTS" and is hereby made a part of said Request for Bids to the same extent as though it were originally therein.

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1. Per specs, there is to be {2} enclosures with {1} well pump each, but if there is not enough gallons per hour there could be {2} additional pumps. How is this to be priced for bidding the project, electrically as far as I am concerned? Bid electrical as outlined and then if more pumps are needed, we will address during project construction meetings.

2. Is there an existing electrical panel to get the power for controls and pumps? If so, is there a plan that shows this? The irrigation is to be "turnkey" including ALL of the electrical work (including wiring, rain sensors, grounding rods, etc.) that is required to make the system operable.

3. Who is responsible for installing the conduit and wiring for the rain sensors and heads? The irrigation is to be "turnkey" including ALL of the electrical work (including wiring, rain sensors, grounding rods, etc.) that is required to make the system operable.

4. Who is to install the controller ground rods and grounding? The irrigation is to be "turnkey" including ALL of the electrical work (including wiring, rain sensors, grounding rods, etc.) that is required to make the system operable.

5. Is there a list of approved general contractors that are bidding this project? No, there is currently no such list. In order to be approved to bid this project, contractors must attend this mandatory Pre-Bid meeting and be pre-qualified by the County.

6. The table of contents page for the specs states the irrigation system is by the owner. The irrigation by owner is a reference to the items provided (drawing sheets and specs) to clarify it is not supplied by the engineer, NOT that the irrigation will be supplied by the owner.

7. The specs (Irrigation 1.03) call for a 65gpm well and pump but the irrigation plan and same spec calls for 67gph. The irrigation system should be designed for 67 gallons per minute.

8. The spec says the county will provide a 2" meter for the irrigation. Which is it, meter or well to be installed or is the meter going to be safety net? We are requesting a well, not meter.

9. Bid form under unit prices - please specify the depth of asphalt and what depth of ABC is to be included as part of this unit price. The unit price for asphalt should be per SY not per LF. 1.5 inches of asphalt in accordance with NCDOT standards and 6 inches compacted aggregate base course with extending the base course 6 inches beyond asphalt trail edge, 1 foot preferred. There is to be woven geotextile fabric between ABC and soil per specs.

10. Please clarify alternates. The drawings reference the basketball court and 10' walking trail as alternates. The specs & bid form do not shown any alternates. Basketball court and walking trail should be "Bid Alternate." The County will prepare a revised bid form.

18. I pulled down the plans for Castle Hayne Park this morning. Starting to work thru the plans and I noticed on Sheet 4 it calls out a 10' Walking Trail as a Bid Alternate. On Sheet 7 that trail is simply labeled 10' Walking Trail (not labeled as a Bid Alternate) Is the part of the trail (as shown on Sheet 4) an alternate section and the section(s) shown on Sheet 7 part of the Base Bid? Or is the Unit Price requested on Page 2 of 3 (Bid Proposal Form) for Asphalt the Bid Alternate since that unit cost is not included in the Total Bid? The entire 10' wide walking trail should be listed as a "Bid Alternate" and be quoted as a unit cost. This way we can construct only the lineal footage of what we can afford at the time. Hopefully we will be able to construct the entire amount of the walking trail, but at least this way (with the unit cost amount) we can calculate the portion to install first with the funds we have in hand. The County will prepare a revised bid form.

19. Is there a soils report available for review? There was not a soils report done for this project, however a geologist did do a seasonal high water table determination and infiltration rate in the location of the stormwater pond as part of the permit requirement. A copy of the report is attached.

20. Can prospective bidders drill or excavate the site prior to bidding? No, equipment will not be allowed on-site prior to awarding contract.

21. Which type of steel bollards will be used from the detail? The owner would like the removable type.

22. Will access be allowed from Madeline Trask Drive? It is the owner's intention to allow construction access from Madeline Trask Drive.

23. What is the extra parking shown on the radius of the driveways to the parking lot shown on sheets 4 and 7? As part of the permitting process, we showed future overflow grass parking. Please disregard these overflow parking areas, no improvements are needed at this time.

24. Do you have specifications for the sand used on the athletic fields? This is covered in the specifications outlining percentages, etc. Sand base shall be all certified coarse sand.

25. How far outside the playing surface of the athletic field's does the field cross section need to extend? 10' outside playing field

26. What portion of the proposed 10' walking trail will be in wooded areas? We originally stated that we would put the trail overlay on an aerial image, but realized that wouldn't really be helpful. We are requesting that you provide two separate prices, one for the trail in the cleared areas and one in the open areas. So please bid the alternate 10" walking trail as detailed below:

- Linear foot cost for greenway (10' walkway) as shown on detail sheet 8 in open areas.
- Linear foot cost for greenway (10' walkway) as shown on detail sheet 8 in wooded areas.

27. Is there a project estimate. No.

Attachments

1. Copy of Infiltration Test Results for Stormwater Design
2. Revised Bid Proposal Form
3. Pre-Bid Sign-in Sheet

INFILTRATION TEST RESULTS FOR STORMWATER DESIGN

PROJECT:

PROPOSED PARKING EXPANSION
CASTLE HAYNE PARK
CASTLE HAYNE ROAD/HIGHWAY 133
WILMINGTON, NORTH CAROLINA 28403

PREPARED FOR:

MR. JEFF PETROFF, P.E.
COASTAL LAND DESIGN, PLLC
WILMINGTON, NORTH CAROLINA

MAY 30, 2014

PREPARED BY:

ATLANTIC GEOSCIENCE INTERNATIONAL, Ltd.
8633 VINTAGE CLUB DRIVE
WILMINGTON, NORTH CAROLINA 28411
(910) 612-4853

Infiltration Test Results for Stormwater Design
Proposed Parking Expansion
Castle Hayne Park
Castle Hayne Road/ Highway 133
Wilmington, North Carolina

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Infiltration Test Results for Stormwater Design

Proposed Parking Lot Expansion
Castle Hayne Park
Castle Hayne Drive/ Hwy 133
Wilmington, North Carolina

May 30, 2014

1.0 PROJECT DESCRIPTION

Atlantic Geoscience International, Ltd. (Atlantic Geoscience) was retained by Coastal Land Design, PLLC to perform infiltration testing of site soils at an undeveloped wooded property located off Madeline Trask Drive and Crowatan Road at the Castle Hayne Park Site in Wilmington, NC. The owner of the property is New Hanover County Parks & Recreation who proposes to increase it's parking capacity on the unimproved parcel.

Atlantic GeoScience, Ltd was requested to evaluate site soil conditions by determining the seasonal high water (SHWT) and performing infiltration testing to determine the hydraulic conductivity rate of the site soils immediately overlying the SHWT.

The approximate location of the soil borings advanced by Atlantic Geoscience, Ltd to enable infiltration testing (K-test) is shown in Figure 1. Soil types encountered in the borings and general soil conditions are described in Section 4.0 below. The results of the hydraulic conductivity testing used to estimate an average infiltration rate (hydraulic conductivity) for site soils is also provided. The results are described in Section 4.2 and Appendix I of this report.

2.0 OBJECTIVE

The objective of Atlantic Geoscience's workscope was to identify soil conditions beneath the subject site and to perform infiltration testing to enable stormwater design by the Project Engineer. Specifically this evaluation was intended to determine:

- Soil types/texture
- Presence and depth of SHWT
- Infiltration rates of site soils simulating the site's ability to infiltrate stormwater

The following methods describe the work performed by Atlantic Geoscience:

3.0 FIELD METHODS

3.1 Augering and Borehole Advancement

On May 7th, 2014, two borings were advanced using a 5.0 centimeter (cm) diameter dove-tail auger in general accordance with ASTM Method D-1586, "Soil Investigation By Auger

Borings". The borings were located at latitude/longitude N 34.32932 and 077.90412 as determined by a Garmin GPSmap 60CSx global position satellite receiver. Samples were collected and logged as the auger was advanced vertically in six inch intervals for geotechnical and soil classification purposes. Hand samples were collected and visually observed for mineralogy, evidence of oxidation or reduction, structural and relic features, and other indicators of the SHWT. The two borings were advanced to evaluate site soils and to conduct infiltration testing. The deepest boring was advanced to approximately 52.5 inches below grade. A second boring nearby was advanced to a depth of 24.0 inches below grade. In the 24.0" deep boring, one vertical hydraulic conductivity test was performed using the Amoozegar Constant Head Permeameter™. The K-test is designed to simulate the site's capability to infiltrate storm water. A uniquely fabricated 5.0 cm diameter cylindrical flat-bottom shaving auger head with a shallow shaving tooth was used to shape the bottom of the borehole. This step creates a smooth plane on the bottom surface of the borehole and is recommended by the technology developer, Dr, Aziz Amoozegar.

3.2 Infiltration Testing Method Using the Constant Head Permeameter

Infiltration testing is routinely performed to simulate stormwater infiltration and determine the hydraulic conductivity (K) of a particular soil strata's ability to transmit water. Infiltration testing (referred to as "K-testing"), involves the measurement of a volume of water moving through a saturated or unsaturated soil column over a given period of time. Darcy's Law describes the basic relationship of water flow through soil relating flux density (V) to the hydraulic conductivity (K) and the gradient of soil water potential (H). Conductivity is a spatially variable but is constant under saturated conditions for any given position in the field at any given time where:

$$V = -K \operatorname{grad} H$$

The permeameter enables insitu measurement of the saturated hydraulic conductivity (Ksat) of a range of soil types. Infiltration testing was performed in accordance with the methods and procedures developed by Dr. Aziz Amoozegar, Professor of Soil Engineering at NC State University, cited in the Compact Constant Head Permeameter, Users Manual, Ksat, Inc.

A 52.0" deep pilot-hole boring was advanced to determine the predominant soil types beneath the subject site. The borehole enabled evaluation of soil structure, bedding, horizons, and SHWT conditions. An additional shallow 5.0 cm diameter boring was advanced to a test depth of 24" below grade for the purpose of conducting the infiltration test. The bottom of the boring used for testing was smoothed with a flat-bottom shaving auger. The Amoozegar permeameter was used for saturating the borehole by releasing approximately 15 liters of water from the reservoir chamber through a water tubed dissipater into the borehole. This process was continued until steady state saturated conditions were observed and an 8.0" column of water remained static in the borehole. Once steady-state saturation was achieved, flow rates through the permeameter flow-measuring chamber were measured at one-minute time intervals. Upon collecting four successive uniform flow readings, steady state saturation was achieved and the test was concluded.

The data generated by the test was reduced using the Glover Equation (shown below) to determine an average Ksat value of soils tested. The raw data collected during the constant head infiltration test is reported in Appendix I.

Glover's Equation: Ksat = AQ

Where: $A = \{\sinh^{-1}(H/r) - [(r/H)^2 + 1]^{1/2} + r/H\} / (2\pi H^2)$
Q= steady state flow rate (cm^3 /min)

4.0 RESULTS OF SITE EVALUATION

Below are the findings of the evaluation to determine site soils, season high water table conditions, and infiltration rates of soils beneath the subject site.

4.1 Soil Classification

Soil samples were visually classified/described in the field in accordance with ASTM-D 2488-84, "Description and Identification of Soils (Visual Procedures)" and in accordance with the Unified Soil Classification System (USCS). The predominant soils encountered were generally classified as fine grained sands (SP). Below is a table listing the soil types and strata encountered in Boring No. 1.

BORING NO. 1 LOG

BORING DEPTH (inches)	DESCRIPTION	MOISTURE CONTENT	OXIDATION/ REDUCTION	USCS CLASS	USDA ASSOCIATION
0 to 12"	Black Fine Loamy SAND	Low	N	SP	Wrightsboro
12" to 18"	Tan-Grey Fine SAND-Slightly Moist	Slightly Moist	N	SP	Wrightsboro
18" to 24"	Grey Fine SAND w/Faint Olive Mottles	Moist	Y	SP	Wrightsboro
24" to 32"	Grey Fine SAND –No Mottles	Moist	N	SP	Wrightsboro
32" to 40"	Grey Fine SAND w/Faint Black Mottled Matrix	Moist	Y	SP	Wrightsboro
40" to 42"	Yellow-Grey Fine SAND w/ Heavy Orange Mottling/Black Mottles/Nodules	Very Moist	Y	SP	Wrightsboro
42" to 46"	Yellow-Grey Fine SAND w/ Bright Orange Mottles/Clayey Sand	Very Moist	Y	SP/SC**	Wrightsboro
46" to 52"	Grey-Blue Fat Clay w Mottles	Wet	Y	CH	Wrightsboro

** = The Seasonal High Water Table was noted from approximately 42" to 43" below grade. The layer shows indications of present saturation and heavy orange mottling with some black sulfide nodules.

For presence of oxidation/reduction, Y=yes, and N=no meaning that either condition is present or neither condition is not present respectively.

A review of the United States Department of Agriculture (USDA) Soil Conservation Service (SCS), Soil Survey of New Hanover County, North Carolina, published in 1977 was performed. The USDA survey indicates that soil underlying the subject site are predominantly characterized as the **Wrightsboro-Onslow-Kenan Association** which are typically slightly poorly drained soils consisting of sand, fine sand, fine sandy loam, loamy fine sand, in upper layers and sandy clay loam, sandy loam, and clay loam subsoils. The Wrightsboro soil series are described as moderately well drained soils within this Association and found on nearly level uplands.

The Wrightsboro series appears to be the predominant soils found on the subject site and are typically fine-grained sands (SP) with some medium-grained sands (SM). Typically, the surface layer is a grayish-brown fine sandy loam approximately 0 to 6 inches thick underlain by a series of pale brown fine sands from 6" to 9" and pale brown-yellow sandy loam with few light gey mottle below a 19" depth. From 24" to 36" the Wrightsbors series is described as brownish-yellow fine sandy loam with pale brown mottles, and slightly sticky. From 36" to 48" is a brown-yellow sandy clay loam with a distinct yellow-red mottles, slightly sticky. From 48" to 65" is grey clay with yellow mottles, some flakes of mica, and reddish-yellow mottles.

Permeability for the Wrightsboro soil series is reported by USDA to be moderate. Permeability ranges from:

0 to 9 inches	2.0 to 6.0 inches/hr
9 to 48 inches	0.6 to 2.0 inches/hr
48 to 65 inches	0.06 to 0.2 inches/hr

Available water capacity is low; and shrink-well potential is low to moderate because of fine grained sediment and clay mineralogy in the matrix. These soils exhibit metal reaction to be moderate and concrete reaction to be high. The USDA states the seasonal high water table for the Wrightsboro Series is at depths of 2.0 to 3.0 feet during December-April.

4.2 Infiltration Test Results

An infiltration test (Ksat test) was performed at the subject site using the methods described above in Section 2.0 "Infiltration Testing Using the Amoozegar Constant Head Permeameter". The test was performed using a borehole depth of 24.0". The borehole was terminated below grade within the un-saturated zone above the SHWT. The borehole was saturated using the constant head permeameter until steady-state conditions were achieved. The infiltration test enabled a determination of soil vertical permeability and effectively simulated stormwater infiltration. The infiltration rate determined at Boring 1 follows:

Δ Estimated Ksat for Boring No. 1 = 0.183 inches/hour

Refer to Appendix I in this report for the "K-Test Computation Data Sheet" containing the relevant data and computations for deriving the average infiltration rate shown above.

4.3 Groundwater Table and SHWT Conditions

From the borings advanced at this site, perched groundwater was noted to be positioned at a depth of approximately 24" to 32" below grade. Recent weeks/months of excessive rainfall could be contributing to the high perched water table conditions noted. A static apparent watertable elevation depth was not observed at the time this boring was advanced, Soils above the perched watertable zone were also observed to be moist.

Faint indications of redoximorphic conditions were present in a zone 18" to 24" below grade. In these fine sands, dull iron oxidation and olive muted mottling was noted . This zone was also noted to be moist likely to recent seasonal rainfall. The SHWT is estimated to be at a depth of 40 to 42" below grade where yellow fine sand layer was noted to have heavy redoximorphic conditions including bright orange mottling. This redoximorphic zone sites immediately on top of a fat dull grey marine clay (CH) with dull grey to black manganese nodules Indicating anaerobic soil conditions.

Our soil evaluation of the subject site determined that the apparent watertable elevation is greater than 2.5 ft. below the site. Review of USDA data shows estimates of the seasonal high water table for the Wrightsboro Series to be at depths of 2.0 to 3.0 feet. Site groundwater elevations may fluctuate throughout the year due to seasonal conditions,

Based on soil morphology, redox conditions, and soil wetness observed in the field, we conclude that the SHWT at the site is approximately 40" to 42" below grade, Indications of faint/minimal mottling and redox conditions above 40" may represent relic zones where the SHWT existed at on time and have been altered due to changes in regional drainage patterns or seasonal rainfall. The higher zones of minimal redox are typically wet during some part of the winter and spring. The soils are typically positioned above a restrictive soil horizon as noted at this site.

5.0 LIMITATIONS

This report has been prepared for exclusive use by Coastal Land Design, PLLC and the Project Architect. The report should not be relied upon by other third party entities. The exploratory and evaluative methods implemented by Atlantic GeoScience, Ltd. in performing this limited evaluation are consistent with geo-scientific, geologic, and geo-engineering standards routinely used in performing limited subsurface exploratory investigations. The subsurface exploration data presented in this report include geotechnical, hydrogeological, and geological conditions encountered during test drilling on the subject site. Use of this data is interpretive by nature and requires estimations, inferences, assumptions and interpolation between known data points. Cross-sections and drill logs illustrating subsurface conditions are for information purposes and should not be relied upon for site development, construction, or subsurface excavation purposes. Subsurface conditions represented in the report are subject to vary laterally and vertically across the site. The test results and data presented in this report represent conditions that were discovered during the time borings were advanced at this site. Site conditions described in this report may vary in the future or be altered due to man-made

activities or occurrences. This includes soil types, bedding thickness, and the elevation of groundwater.

Additional drilling, analyses, testing, modeling, and surveying may be necessary to refine the accuracy of the data provided in this report. The user of this data should do so with caution as subsurface conditions at the site may vary from the findings contained in this report.

6.0 PROFESSIONAL SEALS AND SIGNATURES

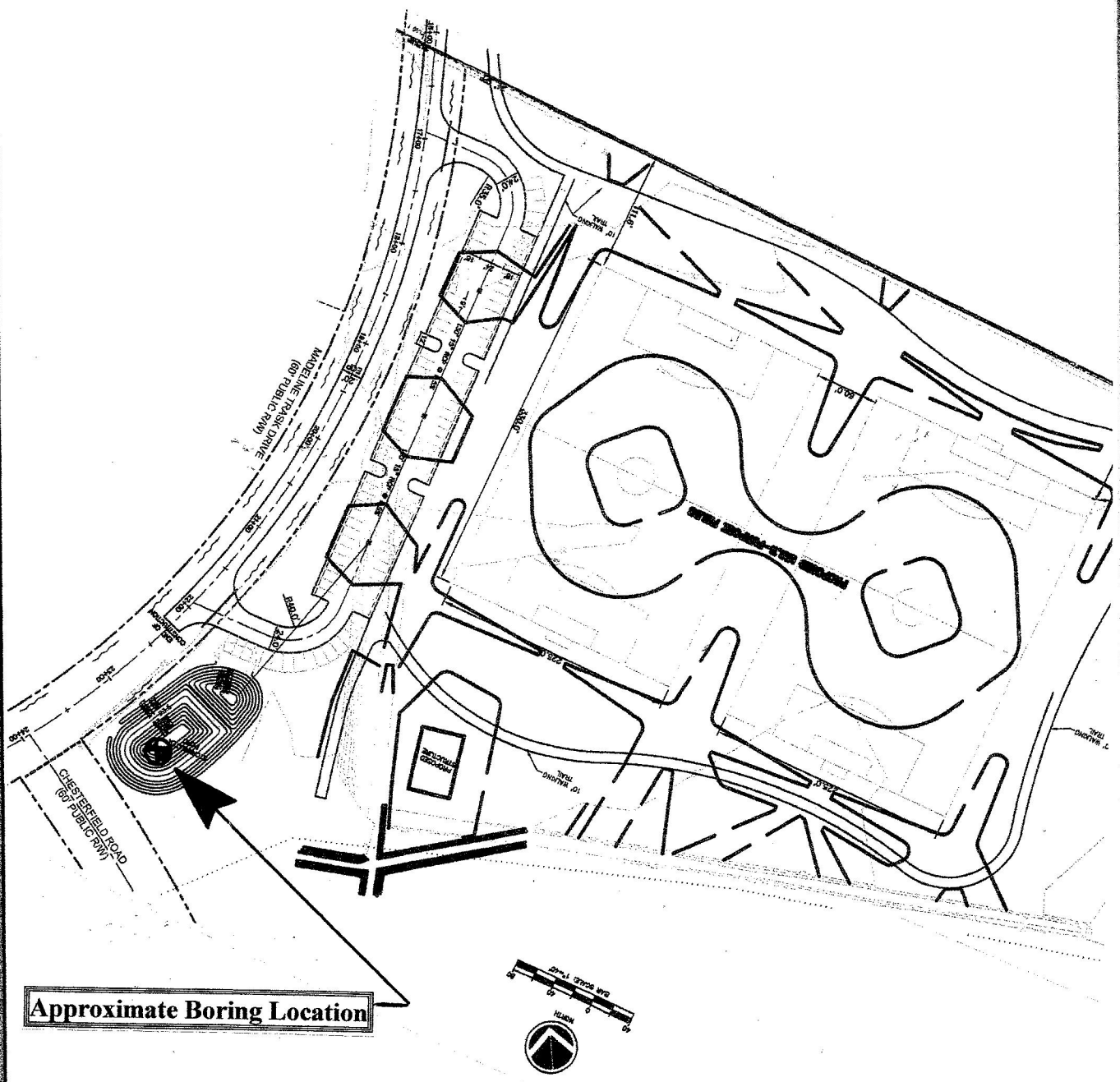
The below parties certify that this report has been prepared under the direct supervision of a Professional Geologist registered in the State of North Carolina. The Professional Geologist in responsible charge has verified the work performed and reported during completion of the work scope including planning, drilling, test pit excavation, field data collection, classification of soils, and technical report preparation.

The data, information, assertions, interpolations, inferences, and statements describing site conditions are believed to be true and accurate. The report has been prepared in accordance with the current standards of practice for geology and geo-sciences.

6-4-14

Rudy A. Smithwick, P.G.
Principal Geologist
NC Stormwater BMP Certification No. 1871

Stephen A. Tyler, P.G.
Project Geologist



Source: Coastal Land Design, PLL, Grading/Drainage Plan, Section Two, Sheet No. 7.

Atlantic GeoScience,Ltd **Wilmington, NC** ***Mining*Exploration*Hydrogeology**	**Infiltration Test Results** **Coastal Land Design, PLLC** **Castle Hayne Park** **Wilmington, North Carolina**		**Site Map w/ Boring Location** **Castle Hayne Park**		Figure: **1**
	Job No.: 2014-018	Date: 5/29/14	Scale: **As Shown**	Drawn By: **RAS/ST**	Ckd. By: **RAS**

K-TEST COMPUTATION DATA SHEET

Site Name: Castle Hayne Park-Parking Expansion

Site Location: Catle Hayne Road/Hwy 133

Wilmington, NC

Test 1

Technician RS/ST

Date: 7-May-14

Weather: Clear, Sunny, 82 F

Borehole Depth: 24.0 inches or 60.96 cm

Borehole Radius (r): 1 inch or 2.54 cm

Distance from Bottom of Reservoir Ruler to top of Borehole: 4.0 inches or 10.16 cm

Distance from Bottom of Reservoir Ruler to bottom of Borehole (D): 28 inches or 71.12 cm

Desired Water Column in Borehole 8.0 inches or 20.32 cm

Constant-Head Tubes Vacuum setting (d): 20.0 in. or 50.8 cm for 14.5" Head (H)

Conversion Factor (C.F.): Measuring Resevoir only (1-on) = 20 cm^2 Measuring and Main (2-on) = 105 cm^2

Time Hour:Minute	Actual d (cm/inches)	H (D-d)	Q (cm^3/min)		Q (cm^3/hour)	Ksat (cm/hour)
			Resevoir Drop (cm) x	C.F. (20 or 105) = E	E / (min/hour) = F	F x Coef A (1/cm^2/hour)
Test 1				CF=105	E/0.0167	Coeficient A: 0.000736
0.01 min	20.0 in	8.0 in	1.27 in or 0.5 cm	52.5	3,144.00	3.66
0.02 min	20.0 in	8.0 in	1.27 in or 0.5 cm	52.5	3,144.00	3.66
0.03 min	20.0 in	8.0 in	1.27 cm or 0.5 cm	52.5	3,144.00	3.66
0.04 min	20.0 in	8.0 in	1.27 in or 0.5 cm	52.5	3,144.00	3.66
						0.464cm/hr or 0.183 in/hr
Test 2	0. 0 in					
0.01 min	0.0 in					
0.02 min	0.0 in					
0.03 min	0.0 in					
0.04 min	0.0 in					

Average Ksat Value **0.464.77 cm/hr** **or** **Ksat= 0.183 in/hr**

Note: Circle which C.F. and increment centemeters (cm) or inches (in) your using for subject test.

Coef. A from H and r data: Table 1 (use r= 2.54) or Table 3 (r=3.5) from CCHP manual.

CASTLE HAYNE PARK IMPROVEMENTS

RFB # 15-0185

Bid Proposal Form

Deadline for Receipt of Bids: 1:00 P.M. EST, Thursday, December 4, 2014

Name of Bidder:_____

Bidder's Address:_____

_____

Bidders Phone Number:_____

Bidder's Email:_____

Bidder's License Number:_____

The undersigned, as bidder, hereby declares that the only person or persons interested in this proposal as principal or principals is or are named herein and that no other person than herein mentioned has any interest in this proposal or in the contract to be entered into; that this proposal is made without connection with any other person, company or parties making a bid or proposal; and that it is in all respects fair and in good faith without collusion or fraud. The bidder further declares that he has examined the site of the work and the contract documents relative thereto, and has read all special provisions furnished prior to the opening of bids; that he has satisfied himself relative to the work to be performed. The bidder further declares that he and his subcontractors have fully complied with NCGS 64, Article 2 in regards to E-Verification as required by Section 2.(c) of Session Law 2013-418, codified as N.C. Gen. Stat. § 143-129(j).

The undersigned, as bidder, proposes and agrees if this bid is accepted, to contract with the New Hanover County for the furnishing of all materials, equipment, and labor necessary to complete the construction of the work described in these documents in full and complete accordance with plans, specifications, and contract documents, and to the full and entire satisfaction of the New Hanover County for the sum of:

Bidder's Name:_____

1. Base Bid $_____
2. Irrigation System $_____
3. Contingency Allowance (5% of line 1 and line 2) $_____

Total Base Bid $_____

Alternates:

1. Basketball Court per plans $_____
2. Cost for 10' walking trail as shown on detail sheet 8 in **OPEN AREAS**. $_____linear foot
3. Cost for 10' walking trail as shown on detail sheet 8 in **WOODED AREAS**. $_____ linear foot

Should the undersigned be required to perform work over and above that required by the Contract Documents, or should he/she be ordered to omit work required by the Contract Documents, he/she will be paid an extra, or shall credit the Owner, as case may be on the basis of unit prices stated herein.

Unit Price: Unsuitable Soil Removal $_____ per cubic yard

Unit Price: Suitable Soil Replacement $_____ per cubic yard

Bidder's Name:_____

The Contractor is hereby notified that the Contract will contain a Liquidated Damages Clause.

A. Performance and Delivery Time:

The Contractor shall begin work on or before the "commence work" date specified in the **NOTICE TO PROCEED** issued by the Owner and as set forth in the plans, specifications, and proposal. All work shall be completed in all events on or before the date set forth in the **NOTICE TO PROCEED.**

B. Liquidated Damages:

Since actual damages for any delay in the completion of the work which the contractor is required to perform under this contract are or will be difficult to determine, the contractor and his /her sureties shall be liable for and shall pay to the Owner the sum of **$200** as fixed and agreed as liquidated damages, and not as penalty for each calendar day of delay from the date stipulated for completion, or as modified in accordance with

Bidder's Name:_____

the terms of this agreement until such work is satisfactorily completed and accepted. Said liquidated damages may be deducted from any payments owed to the contractor by the Owner or collected from the sureties, whichever is deemed expedient by the Owner.

Acknowledgment of Addenda

Addendum No. _____Dated_____ Addendum No. _____Dated_____

Addendum No. _____Dated_____ Addendum No. _____Dated_____

Attachments to Bid Proposal

1. Bid Bond
2. Sample Certificate of Insurance
3. E-Verify Form
4. Identification of Minority/Women Business Participation and Affidavit A or Affidavit B

_____ _____

Signature Printed Name/Title

Date _____

CASTLE HAYNE PARK IMPROVEMENTS
MANDATORY PRE-BID SIGN-IN SHEET
November 12, 2014
RFB # 15-0185

COMPANY NAME	CONTACT PERSON	EMAIL ADDRESS	PHONE NUMBER	MOBILE NUMBER
Wells Brothers Const.	Brad Cockman	bcockman@wellsbrothersconstructioninc.com	910 592 5705	910 592 5705
Luniers construction	Garrius Jones	garrius@luinerconstruction.com	252-747-8124	252-268-1188
HIBBERT ENTERPRISES, INC	PAUL HIBBERT	paul@hib-ent.com	910-742-9692	910-942 9692
TRIANGLE GRADING & PAVING	JUSTIN BAHR	jbahr@triangle grading paving .com	336-584-1745	910.367.2647
Norris Construction Co.	Riggs Norris	Norris Grading @ aol.com	910.763.5035	910-619-1079. ~~910-763-5035~~
Eco Express Solutions, LLC	Leon Felton	leon@ecoexpressllc.com	910-602-1686	910-367-9745
HICAPS, INC	Daniel Hood	daniel@hicaps.com	910-399-3114	336-908-1458
S.T. Wooten Corporation	Jennifer Kiglics	jennifer.kiglics@stwcorp.com	910-762-0325	910-443-0435
ELITE CONSTRUCTION & GRADING	Roy GREGORY	rgregory@eliteconstruction nc.com	910-443-5393	910-443-5393
Charles Hughes Construction	Jason Hurley	jhurley@embarqmail.com	252-566-5040	~~252-560-3049~~ 252-560-3049
Thomas, Corder + Company	Andy Rappold	andyrapp19@yahoo.com	910-791-2733	910-791-2797
SUNLAND BUILDERS	CHARLES HOOKS	Cthooksinc@nc.rr.com	919-920-7682	
SUNLAND BUILDERS	STEPHEN BYRUM	sb_cci@yahoo.com	910-599-5010	
L.M. McLamb & Son	Jimmy Beel	Jimmybeel@LMMCLAMB.com	910 287-6688	910 443 5989

CASTLE HAYNE PARK IMPROVEMENTS
MANDATORY PRE-BID SIGN-IN SHEET
November 12, 2014
RFB # 15-0185

COMPANY NAME	CONTACT PERSON	EMAIL ADDRESS	PHONE NUMBER	MOBILE NUMBER
CWC	Jacob Barton	jbarton@civilworkscontracting.com	910-520-2975	910-520-2975
T+H Construction	Travis Holder	thconstruction.inc@yahoo.com	910-520-8905	910-520-8905
S.T. Wooten Corp.	Len Hunt	Robert.Hunt@stwcorp.com	252-637-4794	252-943-5853
Sierra Structures	Vic Watford	vic@sierrastructures.com	919-471-3500	919-616-1525